

Blast triggered landslide at Steinvik, Tana, Norway

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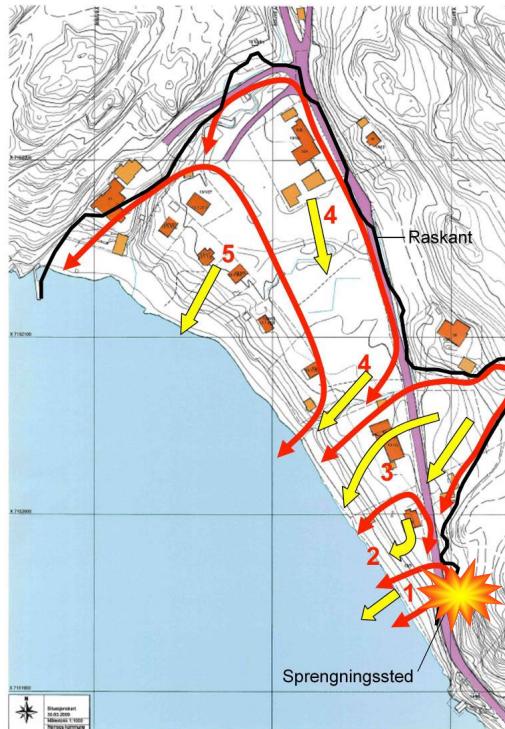
Legislation and regulatory authority, Directorate of Public Roads, Norwegian Public Roads Administration (NPRA)

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Background/perspective – The 2009 Kattmarka slide

- Blasting of rock as part of road construction (near quick clay slope)
- Initial slide started immediately (after ca. 0.5 min) and the entire slide activity lasted 6-10 min after blasting
- Destroyed 4 permanent houses and 6 summer residences. 7 persons rescued unharmed.
- Slide volume $300\text{-}500 \times 10^3 \text{ m}^3$
- A stein block that landed on the slope remoulded the adjacent quick clay material (initial slide!)

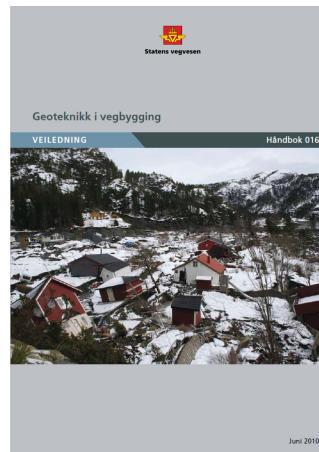


Background – The 2009 Kattmarka slide → Implications on practice

- Key learning points from the slide quickly implemented into regulatory documents
- Geotechnical category (GC) 3 for all projects in quick clay areas => Independent control of projects in GC3
- Limits for blast induced vibrations near quick clay slopes ($\text{cur} < 0,5 \text{ kPa}$)
- Emphasis on ensuring blasted materials do not land on sensitive slopes, improved vibration measurement scheme



2009 (547 pp)



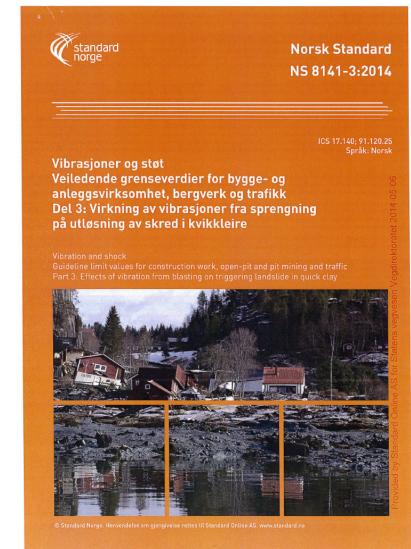
2010 (543 pp)



2022 (653 pp)



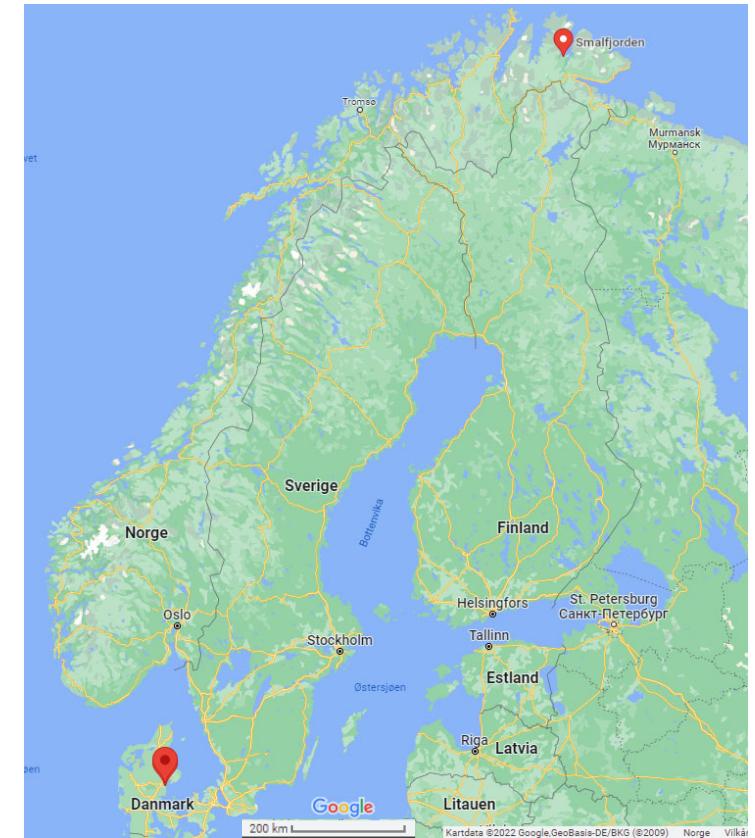
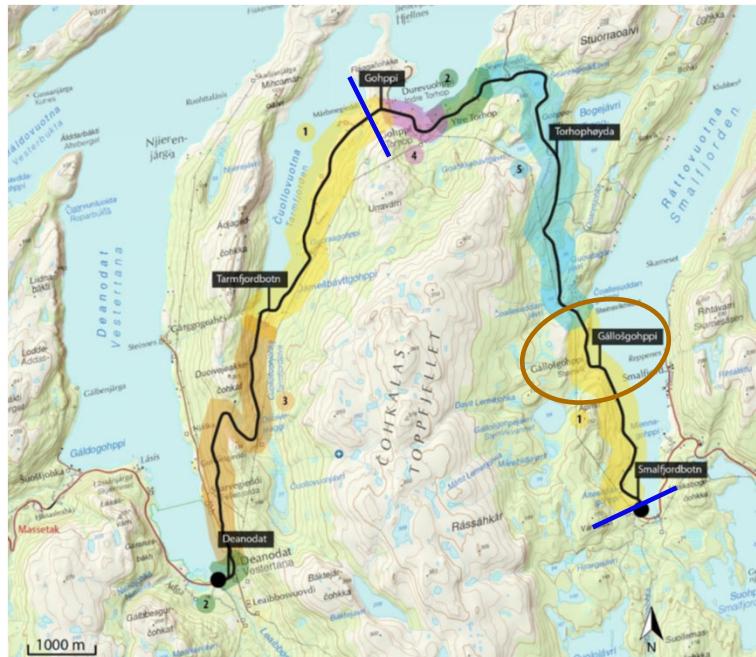
NPRA specifications handbook
for construction of roads (N200)



Norwegian standard
NS 8141-3:2014

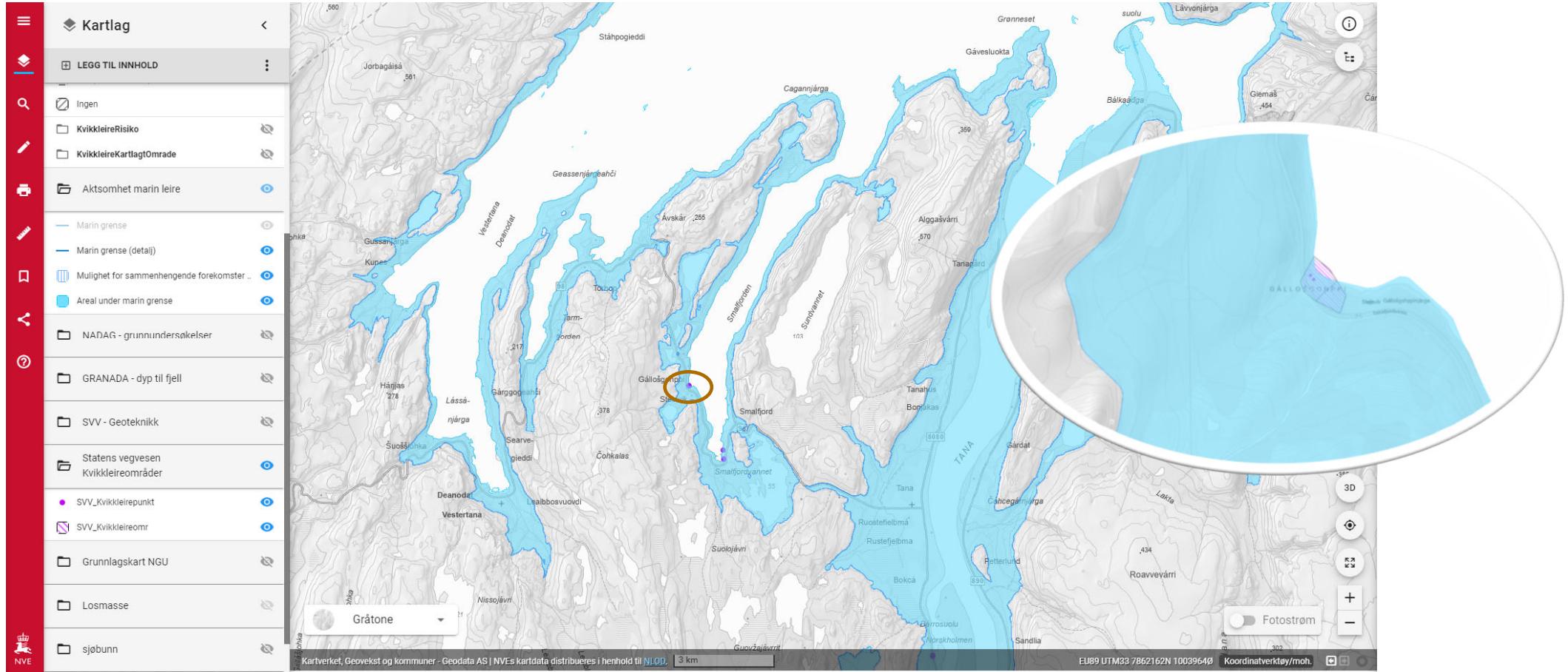
The Project Fv. 98 Torhophøyden – Smalfjordbotn (98 MNOK)

- Located in Tana municipality, Troms and Finnmark county, Norway
- Aim – to improve and upgrade ca. 10 km long road stretch
- Involved extensive blasting of rock masses - a total of 309 000 m³.
- Slide area - Steinvik (Gállošgohppi)



Ground conditions – Areas under marine limit

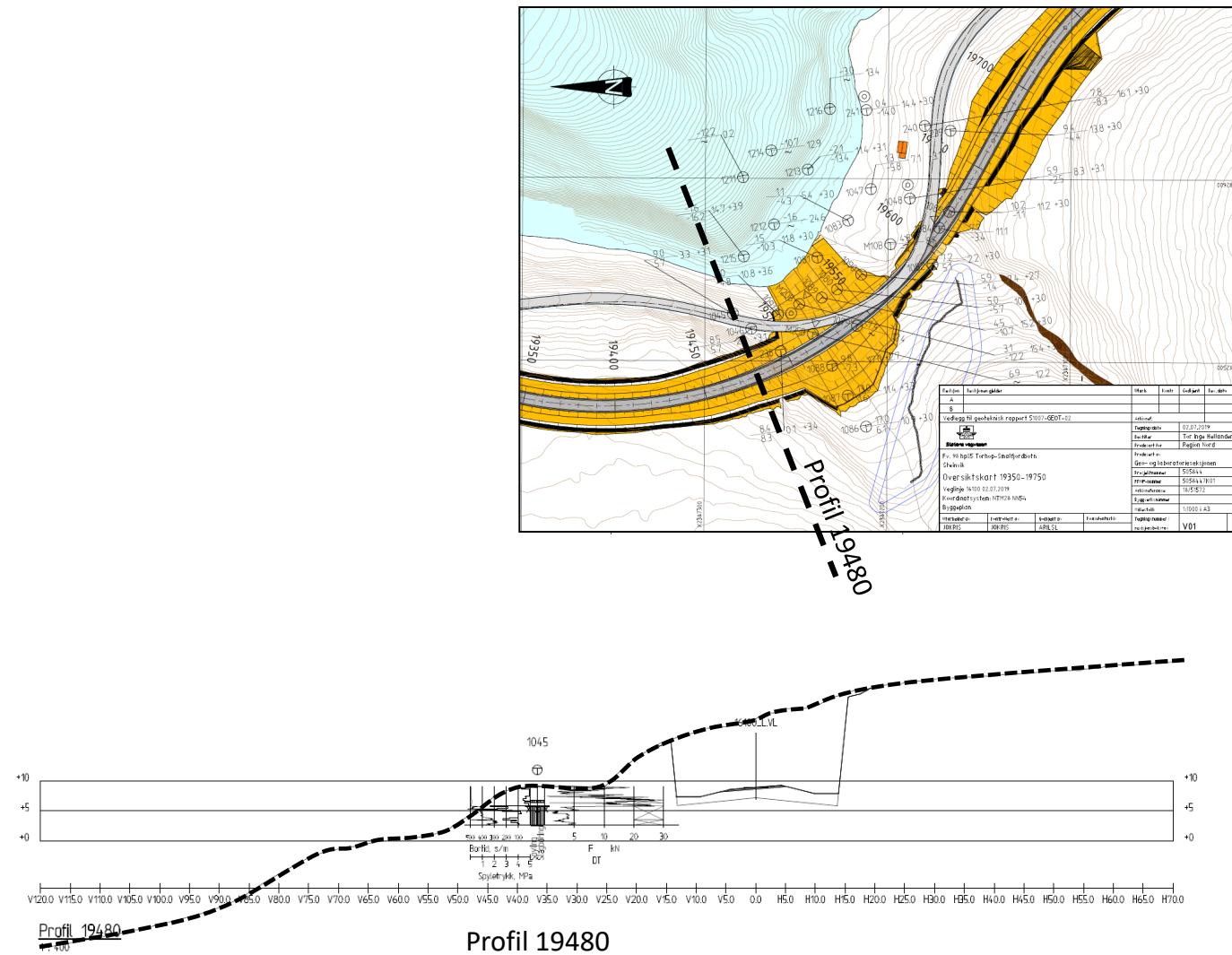
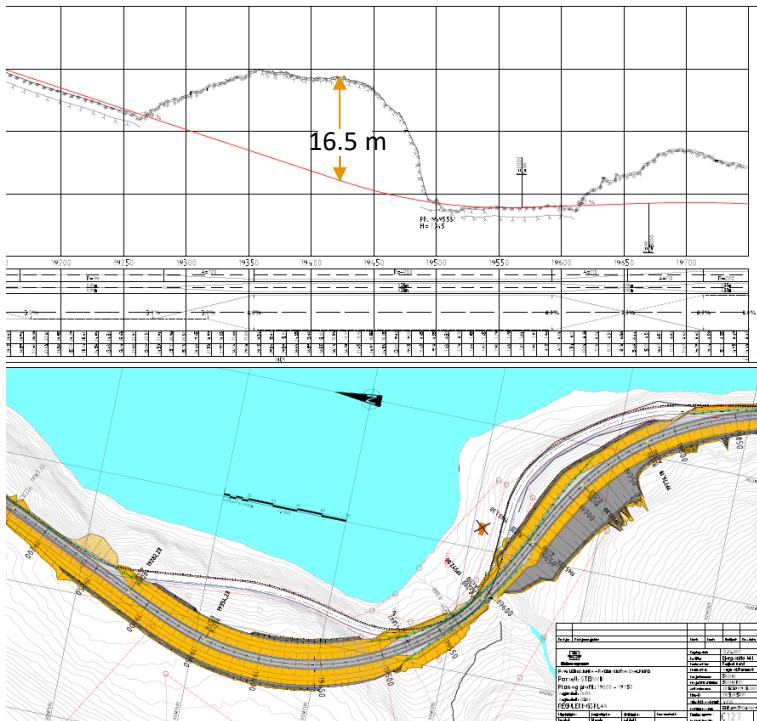
- The prevalence of marine clay (quick clay), is restricted to areas below the marine limit.



<https://temakart.nve.no/tema/kvikkleire>

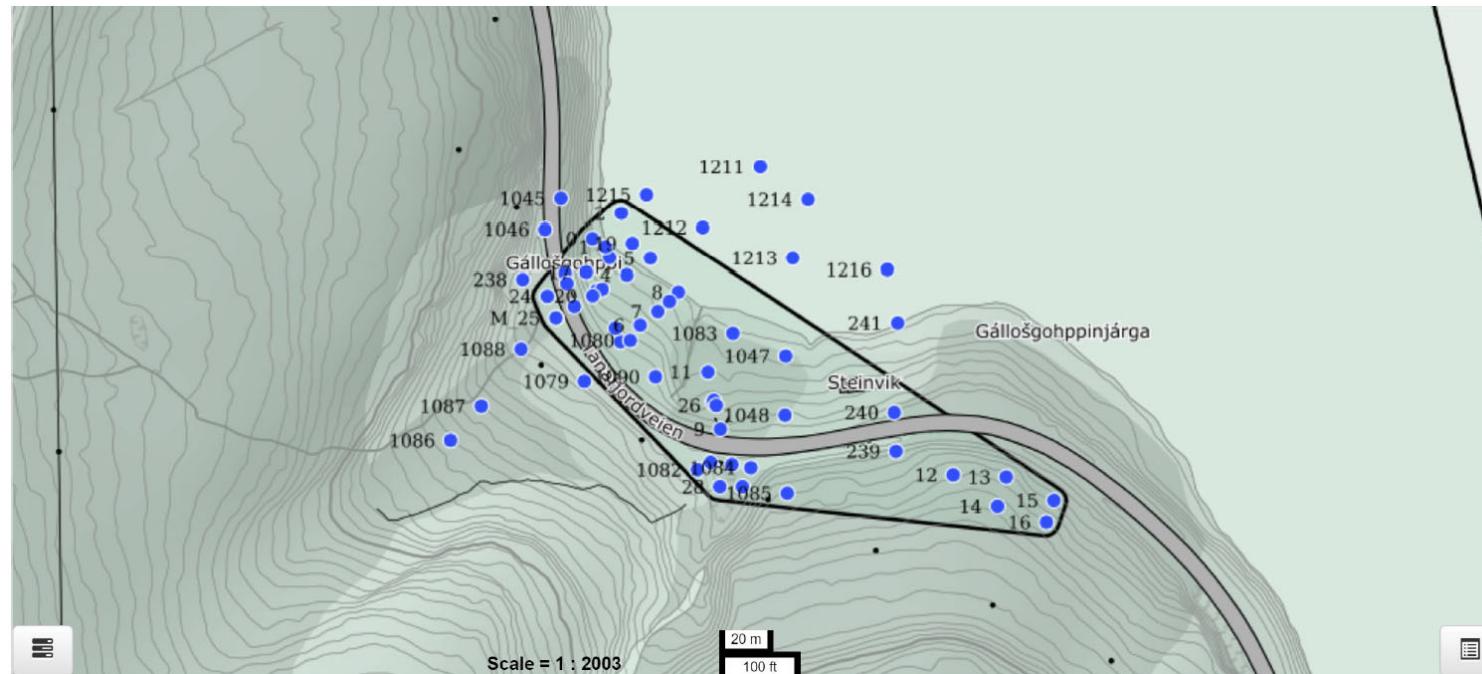
Project plan around Steinvik

- Blasting of rock masses
 - Excavation (unloading of the slope)



Steinvik - soil investigations

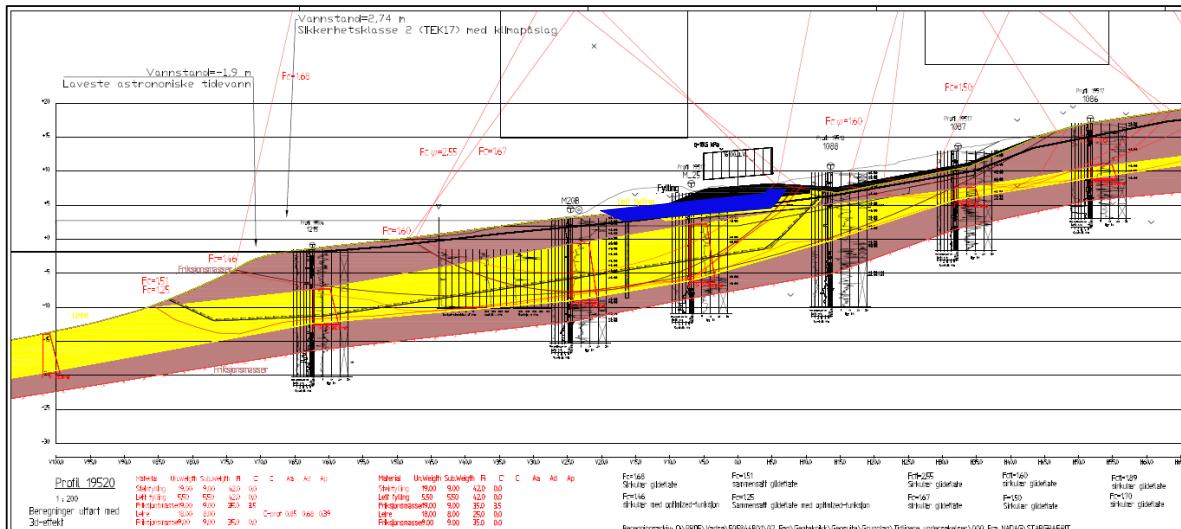
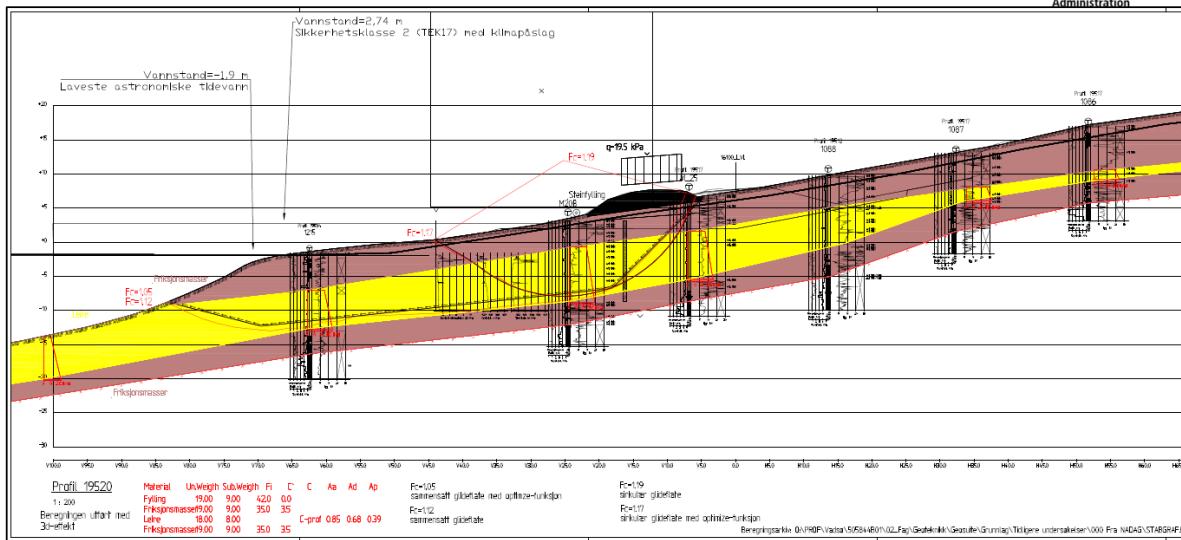
- Steinvik is 150 m wide cove under marine limit
- Total soundings, CPTu, Sampling
- 0 - 25 m thick soil layer – sandy gravel layer (5m) over silty clay and clay of varying shear strength (25 – 37 kPa)
- Contains highly sensitive clay layer – quick clay



Soil investigation points registered in NADAG (National Database for Ground Investigations).
(geo.ngu.no/kart/nadag-avansert/)

Ground conditions – slope stability condition

- Existing stability condition gave a factor of safety lower than the requirements (N200)
- Improvement measures (road moved slightly inwards) – mass replacement with light fillings + terrain unloading
- Design control by independent 3.party => similar improvement measures



Geotechnical execution plan

- Required to avoid placement of materials on the soil slope – whether temporarily or permanently
- Blasting activity shall be carried out in accordance to Norwegian standard NS 8141-3:2014
- Geotechnical engineer presence during the execution phase and approval for any changes

6 Vurderinger

Det skal ikke lagres masser i Steinvik. Hverken midlertidig eller permanent.

Hvis det oppdages at det må graves i leirholdige masser skal geotekniker kontaktes og gi forløpende instruks om hvordan arbeidet skal utføres. Det vil da være aktuelt med enten masseutskifting til fast grunn eller legge duk og jordarmering for å få stabilitet i vegfyllingen.

Sprengningsarbeid som skal utføres i nærheten av Steinvik skal utføres etter [NS8141-3:2014 «Vibrasjoner og støt»](#). Veilegende grenseverdier for bygge- og anleggsvirksomhet, bergverk og trafikk. Del 3: Virkning av vibrasjoner fra sprengning på utløsing av skred i kvikkleire".

8 HMS – FORHOLD

I henhold til byggeherreforskriftene skal det for dette arbeidet lages byggherrens HMS-plan. Dette kapittelet gjelder risiko i forbindelse geotekniske arbeider ved Steinvik.

Ved utførelse av arbeidet må en ta hensyn til fare for utglidninger i Steinvik. Det er derfor et krav at masser ikke får lagres i Steinvik (profil 19500-19630), hverken tilfeldig eller permanent.

I byggefases skal entreprenoren, lage risikovurdering (sikker jobbanalyse) for arbeid i området. Krav om dette skal fremgå av byggherrens SHA-plan.

Blasting and the resulting masses

- Blasting carried out in 3 phases – First ca. 1000 m³ and second ca. 5000 m³
- The 3rd blasting involved mass of ca. 40 000 m³, executed 17.04.21 kl. 18:13 – resulted in 10-15 000 m³ blast material on the road





After blasting

- Activity to open re-open the road started immediately after basting and lasted till kl. 01:00



The slide

- The slide registered at Traffic Control Centre (VTS) @ 03:33 AM –
- Occurred between 01:00 AM and 03:00 AM – at least 6.75 hours after blasting
- The slide took 50 m of the road section.



After the slide

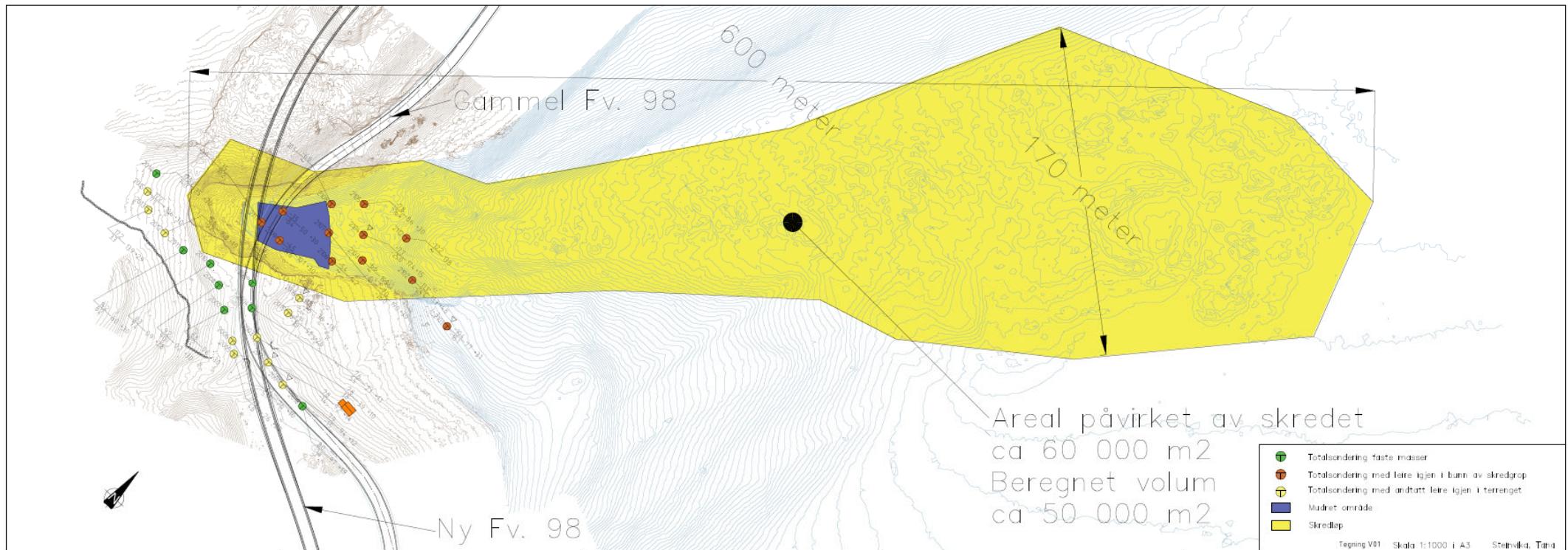
- Road closed for 2 months => detour length of ca. 500 km, direct cost of ca. 18 mill. kNOK
- Slide materials excavated and replaced with crushed rocks



Photos: Frank Martin Ingilæ

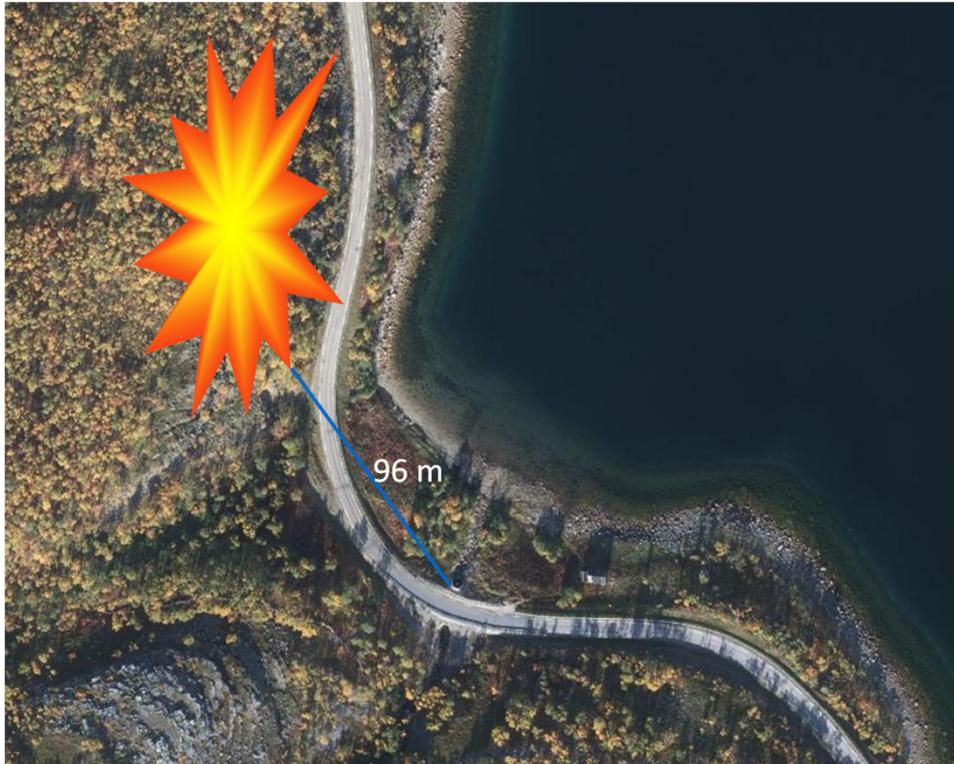
The slide

- Retrogression + runout distance of the slide = 600 m
- Resulting slide volume 60 000m³



Vibration measurement

- 1 Vibration measurement
- Device mounted on a culvert located ca. 90 –100 m from blasting area



<https://kart.finn.no/>

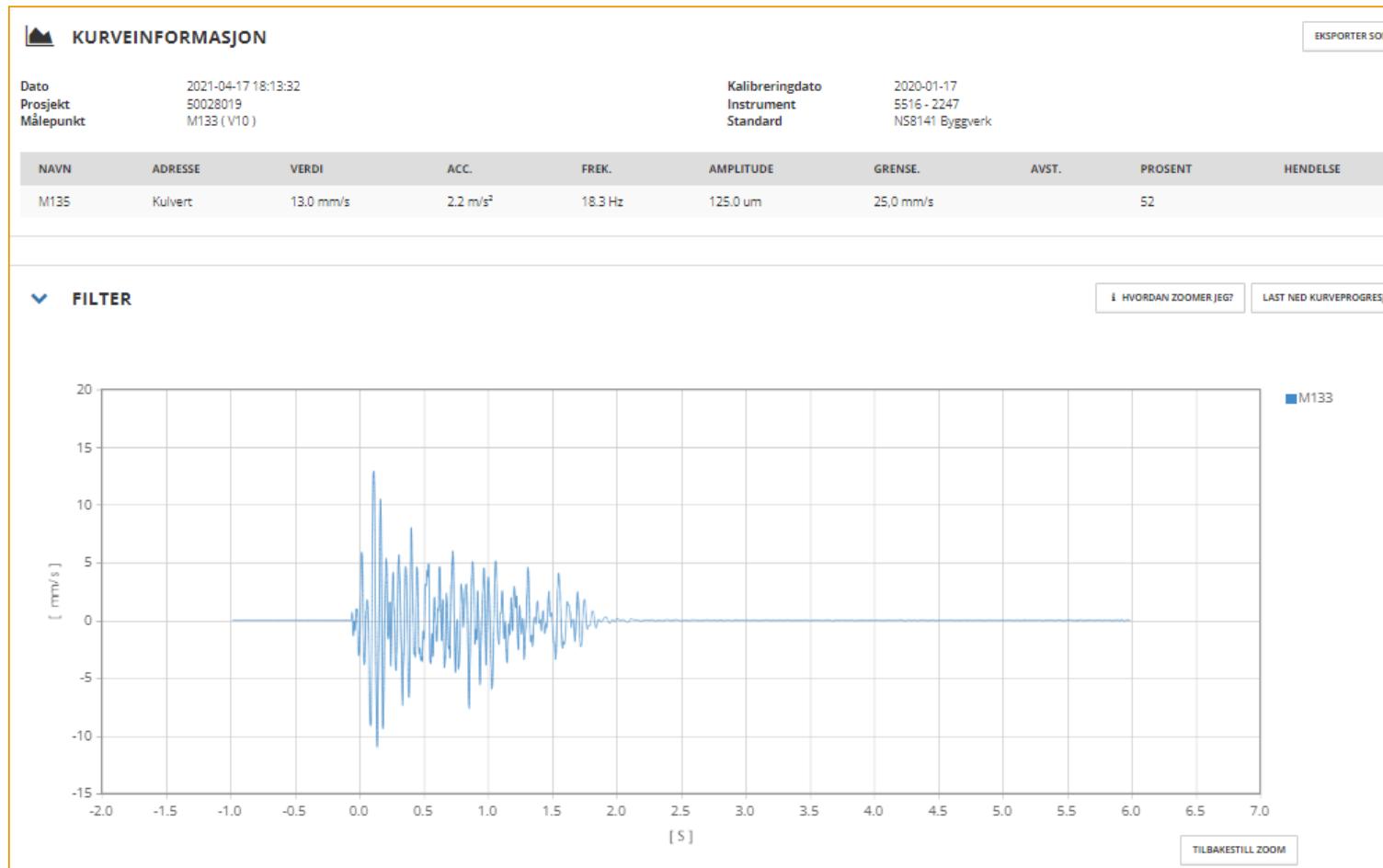


Photos: Frank Martin Ingilæ



Vibrations measurements

- Peak value 13 mm/s, 18:13:32



Critical reflections/discussion points

- In theory, the project had all the essential elements to avoid the slide
 - Relatively small area – straightforward problem
 - Detailed soil investigations and evaluation of stability conditions (3 geotechnical reports)
 - Third party independent design control and verification
 - Enforcement of key requirements from vibration standard and relevant NPRA regulatory documents
 - Knowledge from earlier and relevant incidents – Kattmarka in 2009
- In practice, there were attempts to follow key design elements but with significant deviations from what was expected during design. Shortcoming ? Challenges? Interpretations?
 - Rock type properties in relation to blasting
 - Project execution times/ time needed for careful execution
 - Installation of measuring devices for vibrations
 - Decision on direction of throw of blasted rocks
 - Use of test blasting/earlier blasting than the main one
 - Removing of blast materials without worsening stability conditions
- Important to find explanations and understand where such deviations could emanate from, look into interpretation and use of existing standards.

Final remarks

- Lessons learned from the 2009 Kattmarka slide have been crucial – implementation in relevant regulatory documents have often helped addresses similar problems successfully.
- The slide in Steinvik provides an opportunity to re-examine existing regulations in use and to address any shortcomings so that similar incidents are avoided in the future.
- Analyses of vibration and other relevant data from the project are considered to give key insight into the link between vibrations from blasting activity and stability of sensitive slopes. Re-look at existing limit for vibrations?
- Crucial to address **how** to execute blasting that results in acceptable vibration limit as well as controlled blast materials.
- Clear communications among all involved in project and follow up during execution phase are crucial.
- Successful blasting near sensitive clay slopes is a multidisciplinary problem (geotechnics, geology, rock drilling & blasting). There is a need to create a better overlap/understanding between these professions for practitioners.